

DESIGN CRITERIA NOTES:

1. THE STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE PROVISIONS OF THE INTERNATIONAL BUILDING CODE, 2012 EDITION.
2. DESIGN LOADS ARE AS LISTED BELOW.
- LIVE LOADS:
- | | |
|-----------------------------|---------|
| PRIVATE ROOMS & WARDS | 40 PSF |
| RADIOLOGY, PHYSICAL THERAPY | 75 PSF |
| PUBLIC AREAS | 100 PSF |
| MECHANICAL ROOMS | 150 PSF |
| OR EQUIP. WT. | |
- PARTITIONS IN NON-PUBLIC AREAS 15 PSF
- ROOF 20 PSF
- SUPERIMPOSED DEAD LOADS: MECHANICAL, ELECTRICAL AND CEILING FINISHES WHERE SHOWN ON ARCHITECTURAL 10 PSF AS REQUIRED
3. LOADINGS FOR MECHANICAL ROOMS ARE BASED ON WEIGHTS OF ASSUMED EQUIPMENT AS INDICATED BY THE MECHANICAL DOCUMENTS (INCLUDING THE WEIGHT OF CONCRETE PADS, WHERE INDICATED). ANY CHANGES IN TYPE, SIZE, OR NUMBER OF PIECES OF EQUIPMENT SHALL BE REPORTED TO THE ARCHITECT FOR VERIFICATION OF THE ACCURACY OF SUPPORTING MEMBERS PRIOR TO THE PLACEMENT OF SUCH EQUIPMENT.
4. BASIC DESIGN SNOW LOADS ARE IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE, 2012.
- | | |
|------------------------------------|--------|
| GROUND SNOW LOAD, P_g | 30 PSF |
| FLAT-ROOF SNOW LOAD, P_f | 28 PSF |
| SNOW EXPOSURE FACTOR, C_e | 1.0 |
| SNOW LOAD IMPORTANCE FACTOR, I_s | 1.1 |
| THERMAL FACTOR, C_t | 1.2 |
5. BASIC DESIGN WIND LOADS ARE IN ACCORDANCE WITH ASCE 7-08. DESIGN ASSUMPTIONS ARE AS FOLLOW:
- MEAN RECURRENT INTERVAL = 50 YEARS
BASIC WIND SPEED = 120 MPH
EXPOSURE TYPE FOR EFFECTIVE VELOCITY PRESSURE = B
RISK CATEGORY = II
6. SEISMIC DESIGN - THE STRUCTURE HAS BEEN DESIGNED ACCORDING TO THE INTERNATIONAL BUILDING CODE, 2012.
- | | |
|---|-------|
| SEISMIC IMPORTANCE FACTOR, I_e | 1.25 |
| SEISMIC USE GROUP | III |
| 0.2 SECOND SPECTRAL ACCELERATION, S_s | 0.26g |
| 1.0 SECOND SPECTRAL ACCELERATION, S_1 | 0.07g |
| SITE CLASS | D |
| SEISMIC DESIGN CATEGORY | C |

FOUNDATION NOTES:

1. SPREAD FOOTINGS ARE DESIGNED FOR THE ALLOWABLE NET SOIL BEARING PRESSURE OF 3000 PSF. GENERAL CONTRACTOR TO PROVIDE GEOTECHNICAL REPORT PER SPECIFICATIONS TO VERIFY DESIGN ASSUMPTIONS.
2. PROVIDE CRACK CONTROL JOINTS IN SLABS-ON-GRADE AS INDICATED BY THE SPECIFICATIONS.
3. DO NOT ALLOW SURFACE WATER TO ACCUMULATE AND/OR POND IN EXCAVATIONS. TEMPORARY DEWATERING SYSTEM TO BE USED DURING CONSTRUCTION WILL BE DESIGNED AND INSTALLED BY THE CONTRACTOR IN ACCORDANCE WITH THE RECOMMENDATIONS GIVEN IN THE GEOTECHNICAL REPORT AND THE REQUIREMENTS OF THE GOVERNING BUILDING CODE.

MISCELLANEOUS NOTES:

1. THE DETAILS DESIGNATED AS "TYPICAL DETAILS" APPLY GENERALLY TO THE DRAWINGS IN AREAS WHERE CONDITIONS ARE SIMILAR TO THOSE DESCRIBED IN THE DETAILS. UNLESS NOTED OTHERWISE.
2. ALL DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS, AND DETAILS. DO NOT SCALE THE DRAWINGS.
3. PRINCIPAL OPENINGS, CURBS, AND SLAB DEPRESSIONS ARE SHOWN ON THE DRAWINGS. SEE ARCHITECTURAL, MECH., ELEC., AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSERTS, OTHER OPENINGS, AND SLAB DEPRESSIONS NOT SHOWN. THE CONTRACTOR SHALL PROVIDE FOR ALL OPENINGS, CURBS, AND SLAB DEPRESSIONS WHETHER SHOWN ON STRUCTURAL DRAWINGS OR NOT. SIZE AND LOCATION OF OPENINGS SHALL BE VERIFIED WITH THE MECHANICAL CONTRACTOR. ANY DEVIATION FROM OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO ENGINEER'S ATTENTION FOR APPROVAL PRIOR TO FABRICATION OR INSTALLATION OF STRUCTURAL MEMBERS.
4. THE CONTRACTOR SHALL COMPARE THE STRUCTURAL DRAWINGS WITH THE ARCHIT., MECH., ELEC., PLUMBING, AND CIVIL DRAWINGS TO CONFIRM ALL REQUIREMENTS OF THE WORK. REPORT ANY CONFLICT/DISCREPANCY BETWEEN THE DISCIPLINES TO THE ARCHITECT PRIOR TO FABRICATING OR INSTALLING STRUCTURAL ELEMENTS.
5. THE HORIZONTAL AND VERTICAL DIMENSIONS OF EXISTING STRUCTURES SHALL BE VERIFIED BEFORE WORK IS BEGUN. ANY VARIATION BETWEEN DIMENSIONS SHOWN AND EXISTING DIMENSIONS SHALL BE REPORTED TO THE ARCHITECT.
6. THE CONTRACTOR SHALL INSURE THAT CONSTRUCTION LOADS DO NOT EXCEED THE DESIGN LIVE LOADS INDICATED ON THE STRUCTURAL DRAWINGS AND THAT THESE LOADS ARE NOT PUT ON THE STRUCTURAL MEMBERS PRIOR TO THE TIME THAT THE CONCRETE REACHES THE FULL DESIGN STRENGTH AND ALL FRAMING MEMBERS AND THEIR CONNECTIONS ARE IN PLACE.
7. PROVIDE CHAMFERS AS SPECIFIED AND/OR DETAILED ON THE ARCHITECTURAL DRAWINGS. CHAMFERS HAVE NOT BEEN SHOWN ON THE STRUCTURAL DRAWINGS.

CONCRETE NOTES:

A. CAST IN PLACE CONCRETE:

1. CLASSES OF CONCRETE SHALL BE AS FOLLOWS:

CLASSES OF CONCRETE			
LOCATION	28 DAY f_c (psi)	CONC. TYPE	MAX. NOM. COARSE AGG. SIZE
ALL CONC. U.N.O.	3,000	N.W.C.	1 1/2"
SLAB ON GRADE	3,500	N.W.C.	1 1/2"
PILASTER, WALLS	4,000	N.W.C.	1 1/2"
TOPPING SLABS	4,000	N.W.C.	3/4"

N.W.C. DENOTES NORMAL WEIGHT CONCRETE WITH A MAX. DRY DENSITY = 150 PCF

2. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE AS NOTED BELOW. SEE SECTION 7.7, A01 318-10 FOR CONDITIONS NOT NOTED.

CONCRETE PROTECTION		
WALL FOOTINGS, COLUMN FOOTINGS, AND OTHER CONCRETE PLACED AGAINST SOIL	3"	
WALLS	1" BACKFILLED SIDES 2" NON-BACKFILLED SIDES	
SLABS-ON-GRADE	3" BOTTOM (MINIMUM)	
TOPPING SLABS	1" TOP 1.5" TOP	

3. HORIZONTAL CONSTRUCTION JOINTS SHALL BE PERMITTED ONLY WHERE SHOWN ON THE STRUCTURAL DRAWINGS.

CONCRETE REINFORCEMENT NOTES:

REINFORCING STEEL

- a. CONCRETE REINFORCING BARS SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A615 GRADE 60, EXCEPT AS NOTED. FIELD BENT #3 DOWELS MAY BE ASTM A615, GRADE 40. REINFORCEMENT REQUIRED TO BE WELDED SHALL CONFORM TO ASTM A706, U.N.O.
- b. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. THE FOLLOWING WELDED WIRE FABRIC SHALL BE USED FOR AREAS SPECIFIED BELOW. UNLESS NOTED OTHERWISE ON THE DRAWINGS:
- | | |
|-----------------------------|---------------------|
| 5 INCH SLAB-ON-GRADE | 6 X 8 - W2.9 X W2.9 |
| NON-STRUCTURAL TOP OF SLABS | 6 X 8 - W1.4 X W1.4 |
- FOR ADDITIONAL WWF, IF ANY, SEE FLOOR FRAMING PLANS.
- c. HEADED STUDS AND DEFORMED BAR ANCHORS USED IN FABRICATION OF EMBEDDED ASSEMBLIES SHALL BE WELDED TO THOSE ASSEMBLIES USING A FULL FUSION PROCESS.
- d. REINFORCING BARS MAY BE SPLICED ONLY AS SHOWN ON THE DRAWINGS EXCEPT THAT REINFORCING DESIGNATED AS "CONTINUOUS" SHALL HAVE A CLASS "B" LAP SPLICE (AO 118-08, SECTION 12.15.1). LAP SPLICES OF CONTINUOUS REINFORCING SHALL BE MADE OVER SUPPORTS FOR BOTTOM BARS AND FOR INTERMEDIATE BARS AND AT MIDSPAN FOR TOP BARS. AT EXTERIOR SUPPORTS, TOP AND BOTTOM BARS SHALL BE HOOKED AND INTERMEDIATE BARS SHALL EXTEND TO WITHIN 2" OF EXTERIOR FACE.
- e. HORIZONTAL WALL REINFORCEMENT SHALL BE CONTINUOUS AND SHALL HAVE 90 DEGREE BENDS AND EXTENSIONS AT CORNERS AND INTERSECTIONS AS SHOWN ON TYPICAL WALL CORNER BAR PLACING DETAILS.
- f. LAPS IN WELDED WIRE FABRIC SHALL BE TWO MESH AT SPLICES.
- g. PROVIDE STANDARD BAR CHAIRS WITH PROTECTIVE TIPS AND SPACERS SPACED AS REQUIRED TO PROVIDE SPECIFIED CONCRETE PROTECTION FOR REINFORCEMENT BUT NOT TO EXCEED 3/4" ON CENTER FOR SLABS, BEAMS, AND GRADE BEAMS. PLACE BAR CHAIRS LONGITUDINALLY IN BEAMS DIRECTLY BELOW THE STIRRUPS.

STEEL NOTES:

A. STRUCTURAL STEEL

1. STRUCTURAL STEEL CONSTRUCTION SHALL BE DESIGNED IN ACCORDANCE WITH A.I.S.C. "LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", 2005, U.N.O.
2. STRUCTURAL STEEL SHAPES, PLATES, ETC., SHALL CONFORM TO THE FOLLOWING ASTM DESIGNATIONS, U.N.O.
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|---|-----------------|
| FLOOR/ROOF BEAMS AND GIRDER EXCLUDING W8X10 AND SMALLER | ASTM A992-50 |
| CHANNELS, TEES, ANGLES, BARS, PLATES, W8X10 AND SMALLER BEAMS | ASTM A36 |
| STEEL TUBING (ITS SECTIONS) ($F_y = 45$ KSI) | ASTM A500-GR. B |
| ANCHOR BOLTS | ASTM A499 |
3. CONNECTION BOLTS SHALL CONFORM TO ASTM A325. USE BEARING TYPE BOLTS WITH THREAD ALLOWED ACROSS THE SHEAR PLANE (TYPE N) AT TYPICAL BEAM SHEAR CONNECTIONS, U.N.O. USE TYPE "SC" BOLTS WITH EITHER DIRECT TENSION INDICATOR OR LOAD INDICATOR WASHERS AT ALL BOLTED SLIP CRITICAL CONNECTIONS.
4. A LISTING OF CONNECTIONS CONSIDERED "SLIP CRITICAL" IS AS FOLLOWS:
- BOLTED CONNECTIONS OF TENSION MEMBERS
BOLTS USED IN MOMENT CONNECTIONS
BOLTED SPLICES OF TRUSS TOP AND BOTTOM CHORDS
5. STEEL BEAM CONNECTIONS NOT DETAILED ON THE DRAWINGS SHALL BE DESIGNED BY THE STRUCTURAL STEEL FABRICATOR. BEAM CONNECTIONS SHALL DEVELOP THE END REACTIONS GIVEN ON THE DRAWINGS. WHERE END REACTIONS ARE NOT SPECIFIED, THE BEAM CONNECTION SHALL DEVELOP 80% OF THE BEAMS WEB ALLOWABLE SHEAR CAPACITY. A MINIMUM CONNECTION CAPACITY OF 12 KIPS SHALL BE PROVIDED FOR ALL BEAMS, UNLESS NOTED OTHERWISE BY SPECIFIED REACTION.

THE STRUCTURAL STEEL FABRICATOR SHALL PROVIDE CERTIFICATION BY A PROFESSIONAL ENGINEER, REGISTERED IN THE STATE OF THE PROJECT, THAT THE CONNECTION DESIGN IS IN ACCORDANCE WITH ALL APPLICABLE CODES AND SPECIFICATIONS.

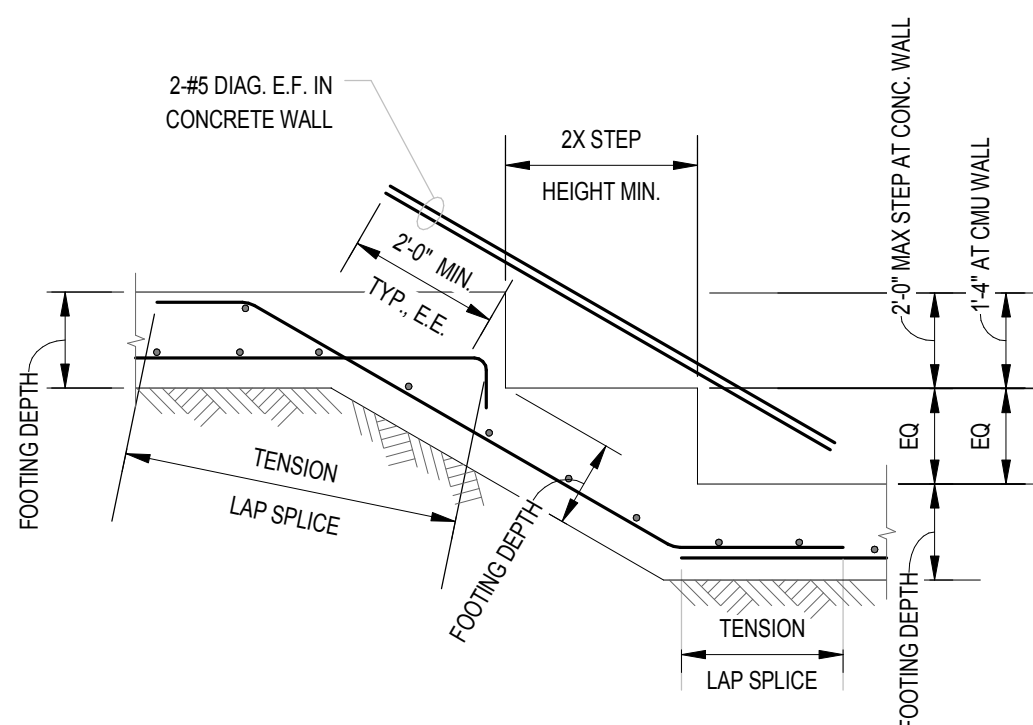
6. FOR ALL HIGH STRENGTH BOLTS, HARDENED WASHERS SHALL BE PROVIDED.
7. GALVANIZING OF STEEL MEMBERS SHALL CONFORM TO ASTM A123.
8. HEADED STUDS AND DEFORMED BAR ANCHORS USED IN FABRICATION OF EMBEDDED ASSEMBLIES SHALL BE WELDED TO THOSE ASSEMBLIES USING A FULL FUSION PROCESS.
9. STEEL BEAMS SHALL BE ERECTED WITH NATURAL CAMBER UP.
10. ANCHOR BOLTS HAVE NOT BEEN DESIGNED FOR ANY SPECIFIC ERECTION FORCES. THE ERECTOR IS RESPONSIBLE FOR ANY AND ALL GUYING AND BRACING REQUIRED TO ERECT THE BUILDING.
11. COMPOSITE BEAMS USING CONCRETE SLAB AS COMPRESSION FLANGE ARE DESIGNED FOR UNSHORED CONSTRUCTION. THE CONTRACTOR SHOULD ANTICIPATE UP TO 5/8" DEFLECTION UNDER WET WEIGHT OF CONCRETE FOR BEAMS WHICH HAVE NO CAMBER SHOWN ON THE STRUCTURAL DRAWINGS.
12. OPEN WEB STEEL JOISTS AND BRIDGING SHALL CONFORM TO THE STANDARDS OF THE STEEL JOIST INSTITUTE. BRIDGING SHALL BE WELDED TO STEEL BEAMS AND ANCHORED TO MASONRY OR CONCRETE WALLS AT THE ENDS, U.N.O.
13. THE RESPONSIBILITY FOR ANY TEMPORARY SHORING OR BRACING DURING THE CONSTRUCTION PHASE BEFORE COMPLETION OF CONNECTION AND POURING OF FLOOR SLAB IS ADDRESSED IN THE SPECIFICATIONS AND IS THE RESPONSIBILITY OF THE CONTRACTOR.
14. IF NOT SHOWN ON DRAWINGS, SUPPORT OF METAL DECK AROUND COLUMN CLOSURE, SCREED PLATES AROUND THE OPENINGS AND EDGE SLAB SHALL BE PROVIDED BY THE CONTRACTOR.
15. DURING CONSTRUCTION, THE ERECTED STRUCTURAL STEEL SHALL NOT PROCEED HIGHER THAN THE CONCRETE CORE CONSTRUCTION. THE CONTRACTOR SHALL MAKE SAFE PROVISIONS FOR STABILIZING THE STEEL STRUCTURE BOTH HORIZONTALLY AND VERTICALLY. THE STABILITY OF THE FRAME DURING ERECTION IS THE CONTRACTOR'S RESPONSIBILITY.

B. WELDING

1. WELDED CONSTRUCTION SHALL CONFORM TO THE AMERICAN WELDING SOCIETY "STRUCTURAL WELDING CODE" D1.1, AWS D1.3-SHEET STEEL, AND AWS D1.4 "REINFORCING STEEL WELDING CODE".
2. ELECTRODES FOR FIELD AND SHOP WELDS OF STRUCTURAL STEEL SHALL BE E70XX, U.N.O.
3. ELECTRODES FOR WELDING OF REINFORCING STEEL SHALL BE E80XX.
4. ELECTRODES FOR WELDING OF SHEET STEEL SHALL CONFORM TO AWS D1.3.
5. WHEN WELDS ARE NOT CALLED-OUT ON DRAWINGS, THEY ARE MINIMUM SIZE CONTINUOUS FILLET WELDS IN ACCORDANCE WITH AWS D1.1. FILLET WELDS NOT SPECIFIED AS TO LENGTH SHALL BE CONTINUOUS.
6. UNLESS NOTED OTHERWISE ON THE DRAWINGS, ALL GROOVE WELDS SHALL BE FULL PENETRATION.
7. ONLY LOW HYDROGEN ELECTRODES SHALL BE USED ON REINFORCING STEEL AND ASTM A992 STEEL.
8. PROVIDE FILLET WELDS AT ALL CONTACT JOINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE STRENGTH OF THE SMALLER MEMBER AT THE JOINT UNLESS DETAILED OTHERWISE ON THE DRAWINGS.

METAL DECK NOTES:

1. METAL DECK SHALL BE GALVANIZED AND SHALL BE PLACED WITH CONTINUOUS SPANS OF THREE OR MORE WHERE POSSIBLE. IN NO CASE SHALL UNSHORED METAL DECK SPANS EXCEED THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS OR DEFLECTION CRITERIA OF SPAN DIVIDED BY 240.
2. ROOF DECK SHALL BE WIDE RIBBED WITH THE FOLLOWING MINIMUM PROPERTIES:
- | |
|--------------------|
| 159-20 GA. |
| $f = 150$ IN/INFT |
| $SP = 139$ IN/INFT |
| $SN = 107$ IN/INFT |
- WELD DECK TO SUPPORTING MEMBERS ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. DECK AND WELDS SHALL HAVE A MINIMUM DESIGN DIAPHRAGM SHEAR CAPACITY OF 200 PLF.
3. DECK UNITS SHALL BE LAPPED ONLY OVER SUPPORTS.

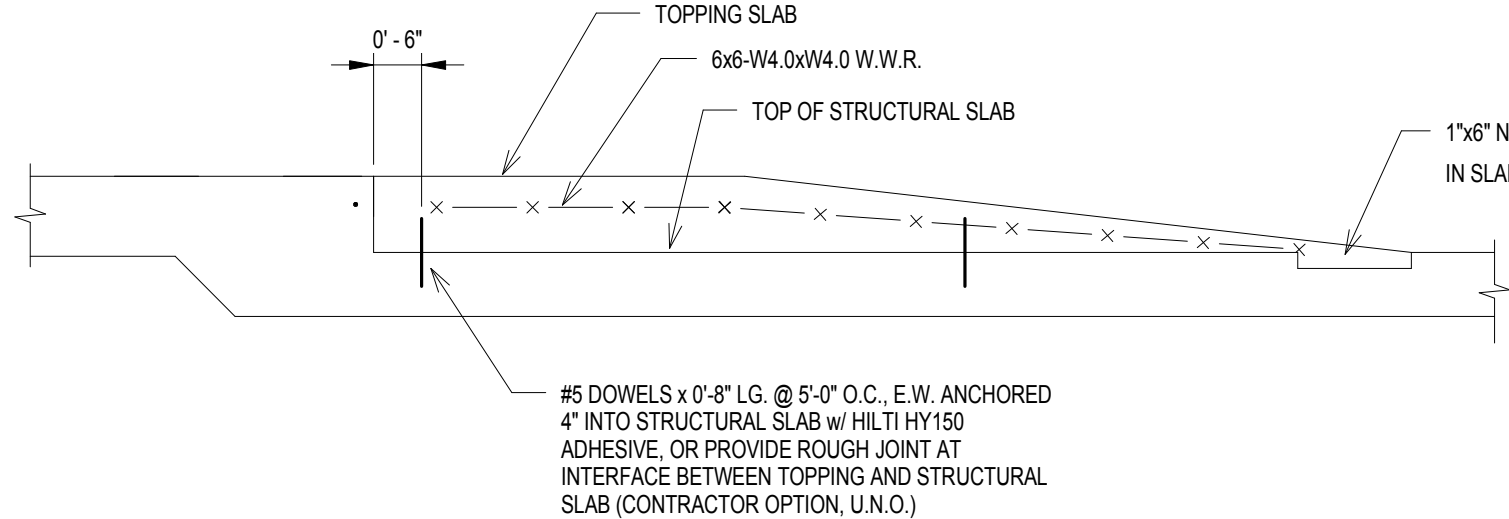


1 STEP IN WALL FOOTING
1/2" = 1'-0"

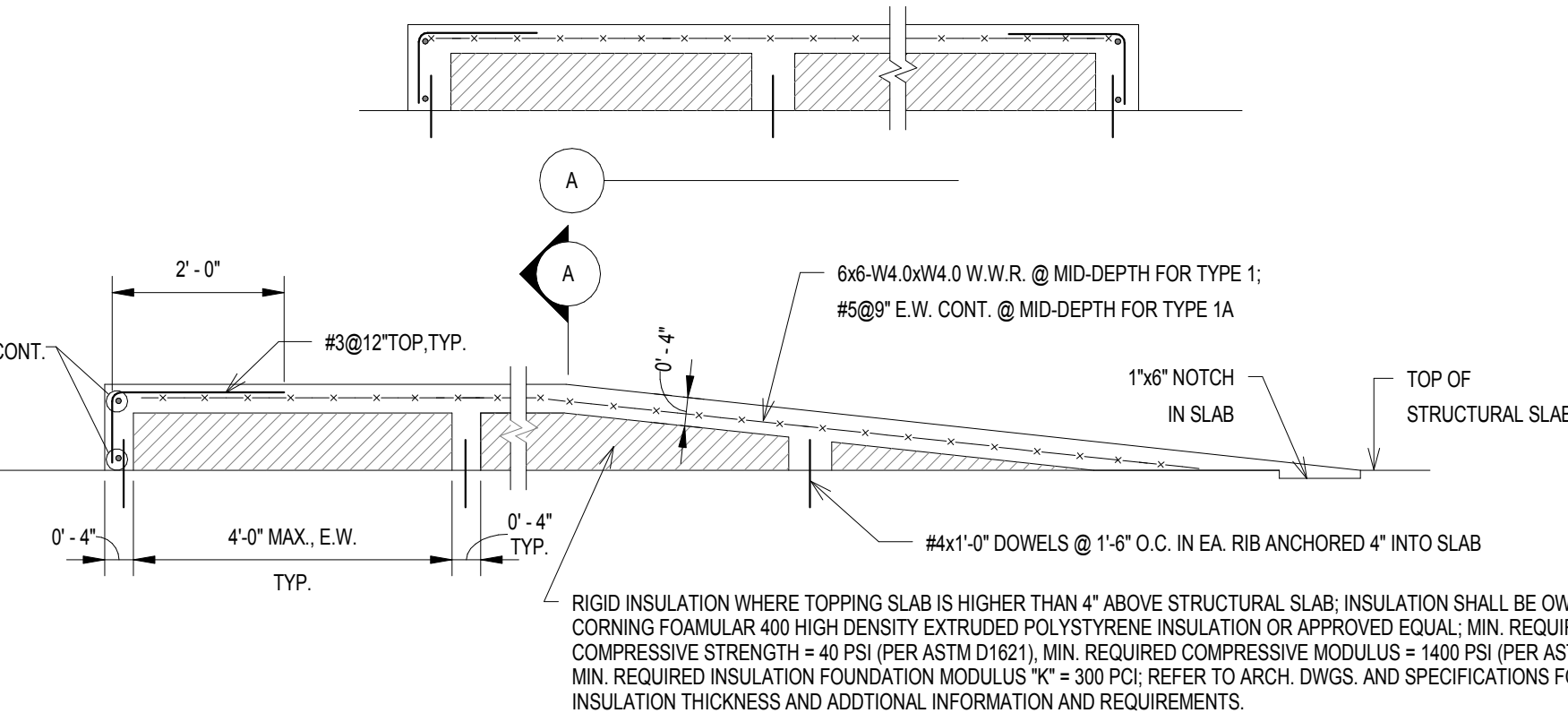
f_c	SPLICE CLASS	BAR SIZE					
		#3	#4	#5	#6	#7	#8
3000	CLASS A 1.0 Ld	13	17	21	27	37	49
	CLASS B 1.3 Ld	17	22	27	35	48	64
4000	CLASS A 1.0 Ld	12	15	18	24	32	42
	CLASS B 1.3 Ld	16	20	23	31	42	55
5000	CLASS A 1.0 Ld	12	13	16	21	29	38
	CLASS B 1.3 Ld	16	17	21	27	38	49
6000	CLASS A 1.0 Ld	12	12	15	19	26	35
	CLASS B 1.3 Ld	16	16	20	25	34	46

- NOTES:
1. USE THE ABOVE DEVELOPMENT LENGTH AND LAP SPLICE TABLE FOR BEAMS, JOISTS, COLUMNS, WALLS, SLABS, ETC. WHEN THE CLEAR SPACE BETWEEN BARS IS GREATER THAN 2 BAR DIAMETERS.
2. WHEN THE CLEAR SPACE BETWEEN BARS IS LESS THAN OR EQUAL TO 2 BAR DIAMETERS, MULTIPLY DEVELOPMENT AND SPLICE LENGTHS LISTED IN TABLE BY 1.43.
3. TENSION DEVELOPMENT LENGTH = L_d . LENGTHS LISTED IN TABLE ARE IN INCHES.
4. PROVIDE LAP SPLICE LENGTH BASED ON THE LARGER BAR BEING LAPPED WHEN BARS OF DIFFERENT SIZES ARE LAP SPLICED.
5. FOR TOP BARS, MULTIPLY THE DEVELOPMENT AND SPLICE LENGTHS BY 1.3.
- TOP BARS ARE DEFINED AS HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE BAR.

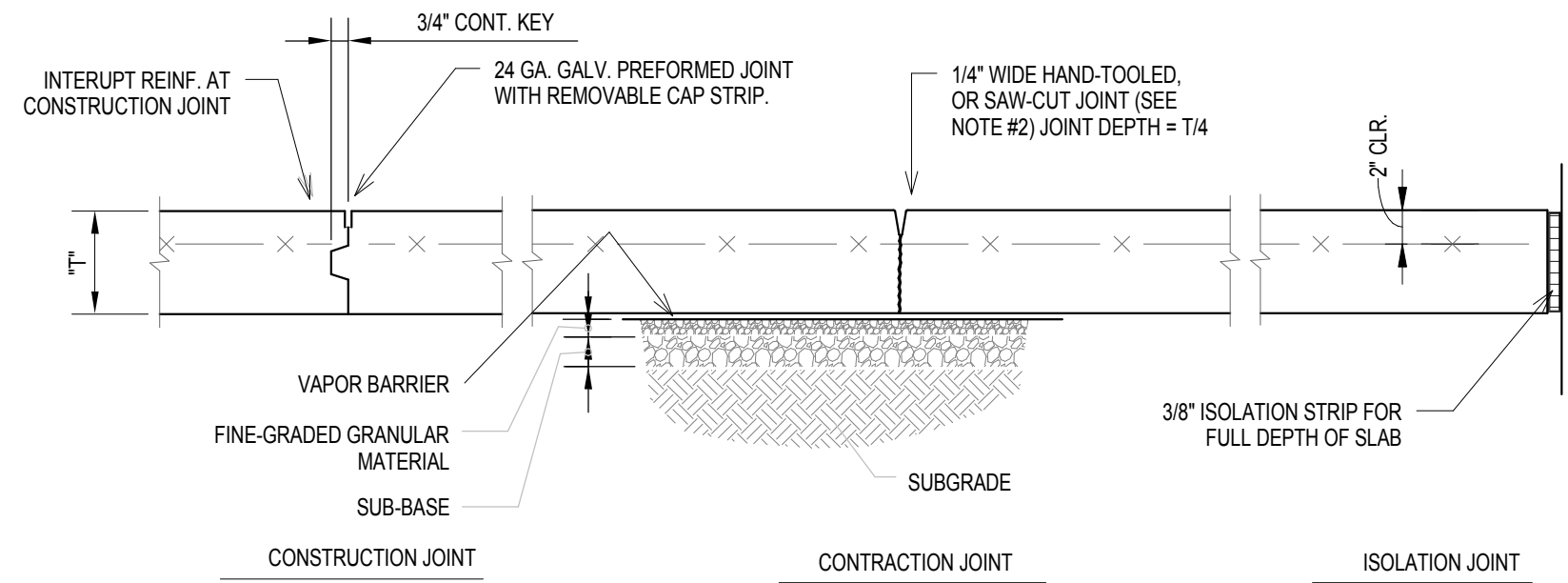
TENSION DEVELOPMENT AND LAP
SPLICE LENGTH FOR N.W.C. (INCHES)
1/2" = 1'-0"



6 TOPPING SLAB
1/2" = 1'-0"

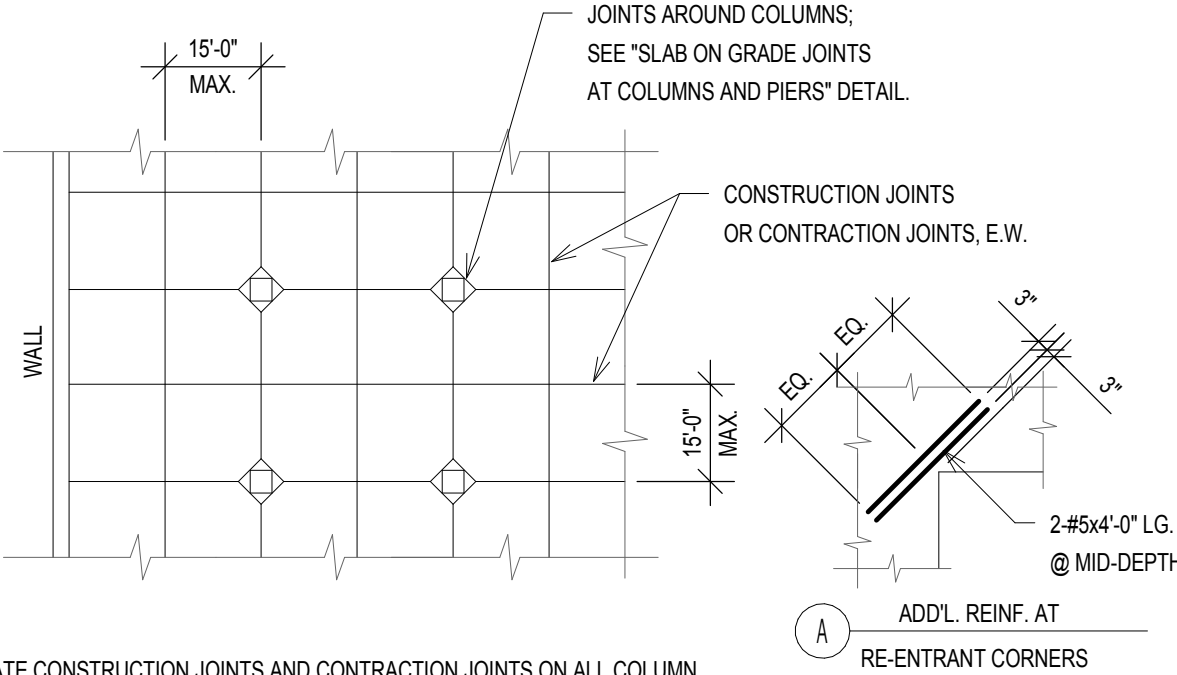


7 TOPPING SLAB
1/2" = 1'-0"



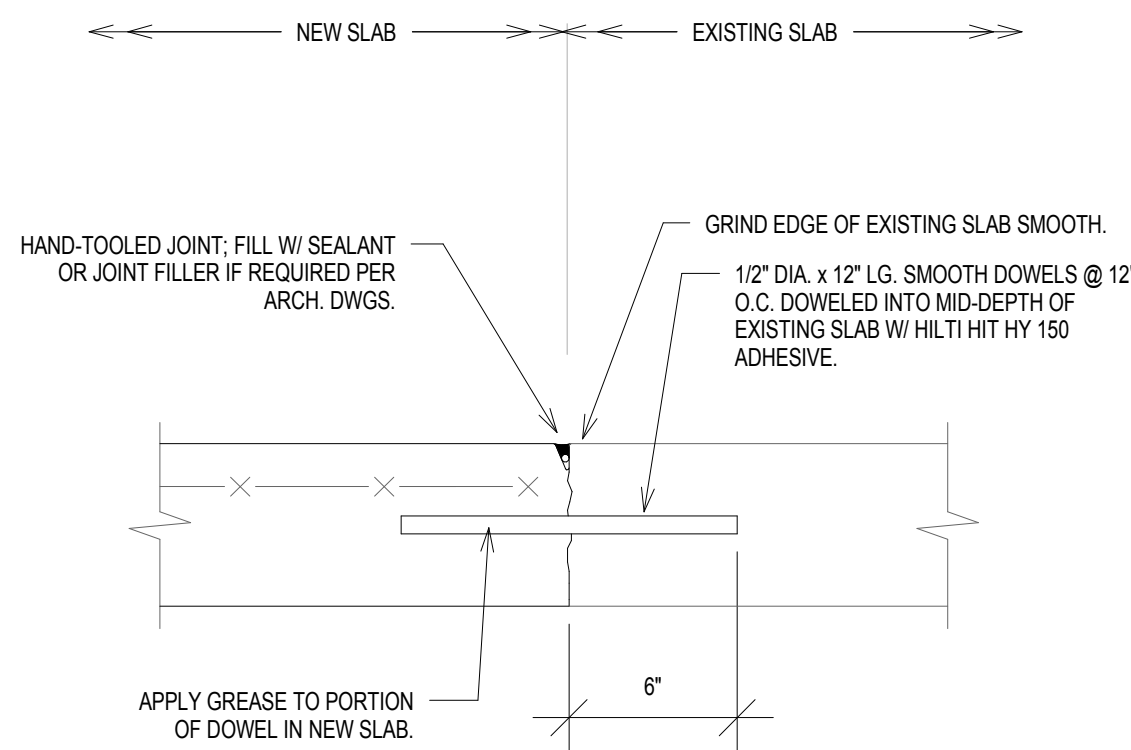
- NOTES:
1. CONTRACTION JOINTS SHALL BE INSTALLED EITHER BY HAND TOOLING, OR BY SAW-CUTTING USING A "SOFF-CUT" ULTRA EARLY ENTRY DRY-CUT SAW OR AN APPROVED EQUAL. JOINTS SHALL BE INSTALLED IMMEDIATELY UPON COMPLETION OF FINISHING OPERATIONS.
2. REFER TO SPECIFICATIONS AND ARCHITECTURAL DRAWINGS FOR JOINT SEALANT REQUIREMENTS (IF ANY).
3. REFER TO SPECIFICATIONS AND TO GEOTECHNICAL DRAWINGS FOR REQUIREMENTS FOR FINE-GRADED GRANULAR MATERIAL, SUB-BASE MATERIAL, AND VAPOR BARRIER (WHERE OCCURRING). REFER TO SPECIFICATIONS FOR SEQUENCE OF INSTALLATION OF FINE-GRADED GRANULAR MATERIAL, SUB-BASE MATERIAL, AND VAPOR BARRIER.

2 SLAB ON GRADE CONSTRUCTION
1/2" = 1'-0"



- NOTES:
1. LOCATE CONSTRUCTION JOINTS AND CONTRACTION JOINTS ON ALL COLUMN GRID LINES AND SUB-DIVIDE AS REQUIRED TO LIMIT MAXIMUM SPACING BETWEEN JOINTS TO MAXIMUM DIMENSIONS INDICATED ABOVE.
2. LOCATE JOINTS IN A PATTERN THAT SUB-DIVIDES SLAB INTO PANELS THAT ARE SQUARE OR RECTANGULAR AND THAT HAVE AN ASPECT RATIO BETWEEN 1.0 (PREFERRED) TO 1.5.
3. PROVIDE JOINTS AT ALL RE-ENTRANT CORNERS OR PROVIDE REINFORCING PER DETAIL "A" AT RE-ENTRANT CORNERS WHERE JOINTS CANNOT OCCUR.

4 SLAB ON GRADE JOINT ARRANGEMENT
1/2" = 1'-0"



NEW SLAB ON GRADE CONNECTION TO
EXISTING SLAB ON GRADE
3/4" = 1'-0"

ADD ALTERNATES

ALT #1
ADD ALTERNATE NO. 1:
Construction and fit out of therapy pool, mechanical area (1-606), pool enclosure (1-603), associated roofing, specialty equipment housed within the pool area, doors 600B, 603, 604B, and daylight 604 are all alternate.

Base bid to include demolition of mechanical unit, slab, and fence (D6), demolition of concrete ramp and canopy (D2, D7, D11), demolition of exterior door near column G4 and subsequent mill of exterior wall.

ABBREVIATIONS USED ON STRUCTURAL DRAWINGS			
A.F.F. ADDL. A.R. L	ABOVE FINISHED FLOOR LINEAR FOOT ANGLE	L.W.C. L.F. L.G. L.H. L.V. L.P.	LIGHT WEIGHT CONCRETE LINEAR FOOT LONG LONG LEG HORIZONTAL LONG LEG VERTICAL LOW POINT
BET. BSMT. B.M. BRG. B or BOT. B.O.D. B.D.G.	BETWEEN BASEMENT BEAM BEARING BOTH SIDES BOTTOM OF DECK BUILDING	MECH. M.E.P. MIN.	MECHANICAL MECHANICAL, ELECTRICAL, PLUMBING MINIMUM
CANT. C.I.P. CL. C.L.R. COL. CONJ. CONN. CONT. C.J.	CANTILEVER CAST-IN-PLACE CENTER LINE CLEAR COLUMN CONCRETE MASONRY UNIT CONNECTION CONTINUOUS CONTROL JOINT	N.S. N.T.S.	NEAR SCALE NOT TO SCALE
DET. DIA. DIM. DIR. D.B.L. DRWG.	DETAIL DIAMETER DIMENSION DIRECTION DOUBLE DRAWING or DRAWINGS	O.C. OPNG. O.D. O.F.	ON CENTER OPENING OUTSIDE DIAMETER OUTSIDE FACE
EA. E.E. E.F. E.W. E.O.D. E.O.S. ELEV. EMBED. EQ. (E) E.J.	EACH EACH END EACH FACE EDGE OF WALL EDGE OF DECK EDGE OF SLAB ELEVATION EQUAL EXISTING EXPANSION JOINT	PEN. PERP. PL. P.T. P.L.F. PSI PSF	PENETRATION PERPENDICULAR PRECAST PLATE POST-TENSIONED POUNDS PER LINEAR FOOT POUNDS PER SQUARE INCH POUNDS PER SQUARE FOOT
F.S. FT. FIN. FL. FTG.	FAR SIDE FEET FINISH FLOOR FOOTING	REIN. REM. RES.D.	REINFORCING or REINFORCE REMAINDER REQUIRED
GALV. GA.	GALVANIZED GAUGE or GAGE	SCHED. SECT. S.W. SIM. S.O.G. SL. SLOPED SPACES SQ. STD.	SCHEDULE SECTION SHORT-WAY SIMILAR SLAB ON GRADE SLOPED SQUARE SQUARE FOOT STANDARD
HGR. HT. H.P. HK. HCR.	HANGER HEIGHT HIGH POINT HOOK HORIZONTAL	STL. STIFF. STRUCT. SYM.	STEEL STIFFENER STRUCTURAL SYMMETRICAL
IN. INT. INV.	INCHES INTERIOR INVERT	TEMP. T T&B T.O.B. T.O.C. T.O.S. T.O.W. TYP.	TEMPORARY TOP TOP AND BOTTOM TOP OF BEAM TOP OF CONCRETE TOP OF STEEL TOP OF WALL TYPICAL
JT. W.W.R. K	JOINT WELDED WIRE 1000 POUNDS	U.N.O. V.I.F. VERT.	UNLESS NOTED OTHERWISE VERIFY IN FIELD VERTICAL
		W.W.F. W.W.R. REINFORCEMENT W/ W.P.	WELDED WIRE FABRIC WELDED WIRE WITH WORK POINT

FULLY SPRINKLERED
ISSUED FOR BID

CONSULTANTS:								PROJECT MANAGER:	Project Number 3619	Scale As indicated	Drawing Title GENERAL NOTES AND TYPICAL DETAILS	Project Title BUILDING 69 PM & R RENOVATION	VA Project Number 542-CSI-203	Office of Facilities Management		
Project Manager BRAY MOONEY CONSULTING	Architect ARRAY HEALTHCARE FACILITIES SOLUTIONS	Structural Engineer WZG STRUCTURAL CONSULTING ENGINEERS	MEP/FP Engineer APOGEE CONSULTING GROUP	Civil Engineer GUIDON DESIGN	Fire Protection Consultant HARRINGTON GROUP	Cost Estimator BRAY MOONEY CONSULTING	Aquatic Consultant ATLANTIC AQUATIC ENGINEERING				Building Number 69					
410 E. 21 STREET CHESTER, PA, 19013 Tel (610) 872-3716	2530 RENAISSANCE BLVD., SUITE 110 KING OF PRUSSIA, PA, 19406 Tel (610) 270-0599	P.O. BOX 24 40 LITTLE ROAD ZIEGLERVILLE, PA, 19422 Tel (610) 287-3194	7330 CHAPEL HILL ROAD, SUITE 202 RALEIGH, NC 27606 Tel (919) 859-7420	2453 N DELAWARE STREET INDIANAPOLIS, IN 46205 Tel (317) 600-6386	7508 E. INDEPENDENCE BLVD., SUITE 116 CHARLOTTE, NC 28277 Tel (704) 531-6077	410 E. 21 STREET CHESTER, PA, 19013 Tel (610) 872-3716	1823 DEEP RUN ROAD PIPERSVILLE, PA, 18947 Tel (215) 766-0409			Approved: Project Director	Location 1400 Black Horse Hill, Coatesville, PA	Drawing Number S001				
Revisions		Date										Date 1/22/2014	Checked RAZ	Drawn NEW	Dwg. 7 of 86	 Department of Veterans Affairs